Chapter Thirty-One

BASIC DESIGN CONTROLS

BUREAU OF DESIGN AND ENVIRONMENT MANUAL

Chapter Thirty-One
BASIC DESIGN CONTROLS

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CHAPTER THIRTY-ONE
BASIC DESIGN CONTROLS

Road design is predicated on many basic controls which establish the overall objective of the highway facility and identify the basic purpose of the highway project. Chapter 31, in combination with Chapter 43, presents those basic controls that impact road design. Chapter 31 includes a discussion on speed, sight distance, traffic volume controls, non-highway controls (e.g., the driver), project scope of work, and the design exception process. The application of these items to a project will impact all elements of road design.

31-1 DEFINITIONS

31-1.01 Qualifying Words

Many qualifying words are used in road design and in this *Manual*. For consistency and uniformity in the application of various design criteria, the following definitions apply:

1. Shall, require, will, must. A mandatory condition. Designers are obligated to adhere to the criteria and applications presented in this context or to perform the evaluation indicated. For the application of geometric design criteria, this *Manual* limits the use of these words.

2. Should, recommend. An advisory condition. Designers are strongly encouraged to follow the criteria and guidance presented in this context, unless there is reasonable justification not to do so.

3. May, could, can, suggest, consider. A permissive condition. Designers are allowed to apply individual judgment and discretion to the criteria when presented in this context. The decision will be based on a case-by-case assessment.

4. Desirable, preferred. An indication that the designer should make every reasonable effort to meet the criteria and that he/she should only use a "lesser" design after due consideration of the "better" design.

5. Ideal. Indicating a standard of perfection (e.g., traffic capacity under "ideal" conditions).

6. Minimum, maximum, upper, lower (limits). Representative of generally accepted limits within the design community but not necessarily suggesting that these limits are inviolable. However, where the criteria presented in this context will not be met, the designer will in many cases need approval.

7. Practical, feasible, cost-effective, reasonable. Advising the designer that the decision to apply the design criteria should be based on a subjective analysis of the anticipated benefits and costs associated with the impacts of the decision. No formal analysis (e.g., cost-effectiveness analysis) is intended, unless otherwise stated.

8. Possible. Indicating that which can be accomplished. Because of its rather restrictive implication, this word will rarely be used in this *Manual* for the application of design criteria.

9. Significant, major. Indicating that the consequences from a given action are obvious to most observers and, in many cases, can be readily measured.

10. Insignificant, minor. Indicating that the consequences from a given action are relatively small and not an important factor in the decision-making for road design.

11. Warranted, justified. Indicating that some well-accepted threshold or set of conditions has been met. As used in this *Manual*, “warranted” or “justified” may apply to either objective or subjective evaluations. Note that, once the warranting threshold has been met, this is an indication that the design treatment should be considered and evaluated – not that the design treatment is automatically required.

12. Standard. Indicating a design value that cannot be violated without severe consequences. This suggestion is generally inconsistent with geometric design criteria. Therefore, “standard” will not be used in this *Manual* to apply to geometric design criteria.

13. Guideline. Indicating a design value that establishes an approximate threshold which should be met if considered practical.

14. Criteria. A term typically used to apply to design values, usually with no suggestion on the criticality of the design value. Because of its basically neutral implication, this *Manual* frequently uses “criteria” to refer to the design values presented.

15. Typical. Indicating a design practice that is most often used in application and which is likely to be the “best” treatment at a given site.

16. Target. If practical, criteria the designer should be striving to meet. However, not meeting these criteria will typically not require a justification.

17. Acceptable. Design criteria that may not meet desirable values, but yet is considered to be reasonable and safe for design purposes.

18. Policy. Indicating IDOT practice which the Department generally expects the designer to follow, unless otherwise justified.

31-1.02 Acronyms

The following are common acronyms for the major national agencies and publications used in road design:

- AASHTO. American Association of State Highway and Transportation Officials.
- FEMA. Federal Emergency Management Agency.
- FHWA. Federal Highway Administration.
- HCM. *Highway Capacity Manual*.
- IDOT. Illinois Department of Transportation.
- ITE. Institute of Transportation Engineers.
- ISTEA. Intermodal Surface Transportation Efficiency Act of 1991.
- MUTCD. *Manual of Uniform Traffic Control Devices*.
- NCHRP. National Cooperative Highway Research Program.
- NHS. National Highway System.
- STP. Surface Transportation Program.
- TEA-21. Transportation Equity Act for the 21st Century.
- TRB. Transportation Research Board.
- TRR. Transportation Research Record.
- USDOT. United States Department of Transportation.

31-2 SPEED

31-2.01 Definitions

1. Design Speed. Design speed is a selected speed used to determine the various geometric design features of the roadway. A design speed is selected for each project which will establish criteria for several design elements including horizontal and vertical curvature, superelevation, and sight distance. Section 31-2.02 discusses the selection of design speed.

2. Low Speed. For geometric design purposes, low speed is defined as 45 mph (70 km/h) or less.

3. High Speed. For geometric design purposes, high speed is defined as 50 mph (80 km/h) or greater.

4. Average Running Speed. Running speed is the average speed of a vehicle over a specified section of highway. It is equal to the distance traveled divided by the running time (the time the vehicle is in motion). The average running speed is the distance summation for all vehicles divided by the running time summation for all vehicles.

5. Average Travel Speed. Average travel speed is the distance summation for all vehicles divided by the total time summation for all vehicles. (Note: Average running speed only includes the time the vehicle is in motion. Therefore, on uninterrupted flow facilities that are not congested, average running speed and average travel speed are equal.)

6. Operating Speed. Operating speed is the speed at which drivers are observed operating their vehicles during free-flow conditions. In practice, the term “operating speed” is commonly used to characterize prevailing vehicular speeds on a highway segment, either through field measurements of speed or through informal field observations. Although no precise percentile is used to define operating speed, it may be assumed to be between the 80th and 90th percentile of actual travel speeds.

7. 85th-Percentile Speed. The 85th-percentile speed is the speed below which 85 percent of vehicles travel on a given highway. The most common application of the value is its use as one of the factors for determining the posted, legal speed limit of a highway section. In most cases, field measurements for the 85th-percentile speed will be conducted during off-peak hours when drivers are free to select their desired speed.

8. Posted Speed Limit. The posted speed limit on State highways is typically based on traffic and engineering investigations, where statutory requirements do not apply. The district Bureau of Operations conducts traffic speed studies on the State highway system. The selection of a posted speed limit is based on several factors:

- the design speed used during project development;
- median type on multilane facilities;
- the 85th-percentile speed and pace speed;*
- highway functional classification and type of area;
- road surface characteristics, grade, alignment, and sight distance;
- type and density of roadside development;
- use of curb and gutter;
- the crash experience during the previous 12 months;
- the need for traffic signal progression; and
- parking practices and pedestrian and bicycle activity.

**Note: Pace speed is the specified increment of spot speed that includes the greatest number of speed measurements.*

9. Legal Speed Limit. Legal speed limits are those set by the Federal government or by the Illinois Statutes which will apply, for example, to those public roads that do not have a posted speed limit.

31-2.02 Design Speed Selection

A design speed is selected for each project which will establish criteria for several geometric design elements including horizontal and vertical curvature, superelevation, cross sectional features, and sight distance. Part V, Design of Highway Types, presents the design speed criteria for new construction and reconstruction projects, 3R non-freeway projects, and 3R freeway projects. In general, the selected design speed is based on the following road design elements:

1. Functional Classification. The higher class facilities (i.e., arterials) are designed with a higher design speed than the lower class facilities (i.e., collectors and locals).

2. Urban/Rural. Design speeds in rural areas are generally higher than those in urban areas. This is consistent with the typically fewer constraints in rural areas (e.g., less development).

3. Terrain. The flatter the terrain, the higher the selected design speed can be. This is consistent with the typically higher construction costs associated with more rugged terrain.

4. Traffic Volumes. On some facilities (e.g., unmarked rural collectors), the design speed varies by traffic volumes; i.e., as traffic volumes increase, higher design speeds are used.

For geometric design application, the relationship between these road design elements and the selected design speed reflects general cost-effective considerations. For example, the higher the traffic volumes, the more benefits to the traveling public from a higher design speed.

In addition to the above, the selected design speed should equal or exceed the anticipated posted/regulatory speed limit of the facility after construction. This applies to all projects. The posted speed limit will be determined based on actual operating speeds of the completed facility and on several factors not directly related to the project design speed. Therefore, to avoid a potential conflict, the designer should, early in project development, coordinate the design speed selection with the district Bureau of Operations to assist in predicting the posted speed limit of the completed facility. If the proposed design speed will be less than the predicted posted speed limit, the designer must choose one of the following approaches:

- increase the project design speed to equal the anticipated posted speed limit,
- post the project with a legal speed limit equal to the design speed, or
- seek a design exception.

In selecting a design speed, the designer should avoid artificially selecting a design speed low enough to eliminate any design exceptions. For example, if the IDOT criteria yield a design speed of 60 mph (100 km/h) and one or more geometric features are adequate only for 55 mph (90 km/h), the project design speed should be 60 mph (100 km/h) and <u>not</u> 55 mph (90 km/h). The designer will then be required to seek design exceptions for the 55 mph (90 km/h) geometric features.

31-3 SIGHT DISTANCE

31-3.01 Stopping Sight Distance

31-3.01(a) Theoretical Discussion (Passenger Cars)

Stopping sight distance (SSD) is the sum of the distance traveled during a driver's perception/reaction (or brake reaction) time and the distance traveled while decelerating to a stop. To calculate SSD, the following formulas are used:

$$SSD = 1.47\ Vt + 1.075\ \frac{V^2}{a} \qquad \text{(US Customary) Equation 31-3.1}$$

$$SSD = \frac{Vt}{3.6} + 0.039\ \frac{V^2}{a} \qquad \text{(Metric) Equation 31-3.1}$$

where:

where:	SSD	=	stopping sight distance, ft (m)
	V	=	design speed, mph (km/h)
	t	=	brake reaction time, 2.5 seconds
	a	=	driver deceleration, ft/s^2 (m/s^2)

For calculating adjusted SSD for downgrades, see Equation 31-3.2.

The following briefly discusses the theoretical rationale for each assumption within the SSD model for passenger cars:

1. Brake Reaction Time. This is the time interval between when the obstacle in the road can first be physically seen and when the driver first applies the brakes. Based on several studies of observed driver reactions, the assumed value is 2.5 seconds. This time is considered adequate for approximately 90% of drivers in simple to moderately complex highway environments.

2. Braking Action. The braking action is based on the driver's ability to decelerate the vehicle while staying within the travel lane and maintaining steering control during the braking maneuver. A deceleration rate of 11.2 ft/s^2 (3.4 m/s^2) is considered comfortable for 90 percent of drivers for passenger cars.

3. Speed. The highway design speed is used to determine the initial driver speed.

AASHTO's *A Policy on Geometric Design of Highways and Streets* presents additional information on the assumptions used to develop the SSD model.

31-3.01(b) Passenger Cars (Level Grade)

Figure 31-3A provides stopping sight distances for passenger cars on grades less than 3%. When applying the SSD values for passenger cars, the height of eye is assumed to be 3.5 ft (1080 mm) and the height of object 2 ft (600 mm). Except as noted in the following subsections, the SSD values in Figure 31-3A apply to all projects.

31-3.01(c) Trucks

The passenger SSD in Figure 31-3A are not designed for truck operations. In general, trucks require longer SSD for a given speed than passenger vehicles. However, truck's higher height of eye (7.6 ft (2330 mm)) and driver experience tends to balance the need for additional stopping lengths for trucks than those for passenger cars (e.g., the truck driver can generally see further beyond a crest vertical curve). Consequently, separate truck SSD are generally not used in highway design. However, the designer still should consider providing longer SSD at the following sites:

- weigh stations;
- rest areas;
- in the vicinity of truck terminals;
- industrial parks;
- coal mining and quarry areas;
- where horizontal sight restrictions occur on downgrades;
- highway/railroad grade crossings on high-volume truck routes (e.g., truck DDHV of 250 or greater);
- other facilities with high truck traffic (e.g., routes with truck DDHV of 250 or greater); and
- locations that have a high incidence of truck crashes.

US Customary					Metric				
Design Speed (mph)	Brake[1] Reaction Distance (ft)	Braking[2] Distance On Level (ft)	Stopping Sight Distance		Design Speed (km/h)	Brake[1] Reaction Distance (m)	Braking[2] Distance On Level (m)	Stopping Sight Distance	
			Calculated (ft)	Design (ft)				Calculated (m)	Design (m)
30	110.3	86.4	196.7	200	50	34.8	28.7	63.5	64
35	128.6	117.6	246.2	250	60	41.7	41.3	83.0	83
40	147.0	153.6	300.6	305	70	48.7	56.2	104.9	105
45	165.4	194.4	359.8	360	80	55.6	73.4	129.0	129
50	183.8	240.0	423.8	425	90	62.6	92.9	155.5	156
55	202.1	290.3	492.4	495	100	69.5	114.7	184.2	185
60	220.5	345.5	566.0	570	110	76.5	138.8	215.3	216
65	238.9	405.5	644.4	645					
70	257.3	470.3	727.6	730					

Notes:

1. *Brake reaction distance based on a time of 2.5 s.*
2. *Driver deceleration based on a rate of 11.2 ft/s^2 (3.4 m/s^2).*

STOPPING SIGHT DISTANCE
(Passenger Cars – Level Grade)

Figure 31-3A

31-3.01(d) Downgrade-Adjusted SSD

The longitudinal gradient of the roadway impacts the distance needed for vehicles to brake to a stop. IDOT practice is to only consider the grade adjustment for downgrades, which increases braking distances. Equation 31-3.1 is modified as follows to calculate the adjusted SSD for downgrades:

$$d = \frac{V^2}{30\left(\frac{a}{32.2} \pm G\right)}$$ (US Customary) Equation 31-3.2

$$d = \frac{V^2}{254\left(\frac{a}{9.81} \pm G\right)}$$ (Metric) Equation 31-3.2

where:
- SSD = stopping sight distance, ft (m)
- V = design speed, mph (km/h)
- t = brake reaction time, 2.5 seconds
- a = driver deceleration, ft/s^2 (m/s^2)
- G = grade expressed as a decimal. Downgrades are expressed as a negative.

Figure 31-3B presents the downgrade SSDs for passenger cars. The designer should make a reasonable effort to meet these SSD values when downgrades are 3% or steeper. However, the grade-adjusted SSD values do not require a design exception when not met.

31-3.02 Decision Sight Distance

31-3.02(a) Theoretical Discussion

At some sites, drivers may be required to make decisions where the highway environment is difficult to perceive or where unexpected maneuvers are required. These are areas of concentrated demand where the roadway elements, traffic volumes, and traffic control devices may all compete for the driver's attention. This relatively complex environment may increase the required driver perception/reaction time beyond that provided by the SSD values (2.5 seconds) and, in some locations, the desired vehicular maneuver may be a speed/path/direction change rather than a stop. At these locations, the designer should consider providing decision sight distance to provide an additional margin of safety. The various avoidance maneuvers assumed in the development of Figure 31-3C are:

US Customary								
SSD FOR DOWNGRADES (ft)								
Design Speed (mph)	(3%)	(4%)	(5%)	(6%)	(7%)	(8%)	(9%)	(10%)
30	205	210	215	215	220	225	230	235
35	260	265	270	275	280	285	290	295
40	315	325	330	335	340	350	355	365
45	380	385	395	400	410	420	430	440
50	450	455	465	475	485	495	510	525
55	520	530	545	555	570	580	595	610
60	600	615	625	640	655	670	690	705
65	685	700	715	730	750	765	790	810
70	775	790	810	825	850	870	895	920
Metric								
SSD FOR DOWNGRADES (m)								
Design Speed (km/h)	(3%)	(4%)	(5%)	(6%)	(7%)	(8%)	(9%)	(10%)
50	66	67	68	70	71	72	74	75
60	87	88	90	92	93	95	97	100
70	110	112	114	116	119	122	124	127
80	136	138	141	144	147	151	154	158
90	164	167	171	174	178	183	187	192
100	194	198	203	207	212	218	223	230
110	227	232	238	243	249	256	263	270

Notes:

1. *Calculated SDDs are not shown. Values in table have been determined by using Equation 31-3.2 and rounding up to the next highest 5 ft (1 m) increment.*

2. *For grades less than 3%, no adjustment is necessary; i.e., use the level SSD values (Figure 31-3A).*

3. *For grades intermediate between table values, use a straight-line interpolation to determine the SSD or use Equation 31-3.2 and round up to the next highest 5 ft (1 m) increment.*

STOPPING SIGHT DISTANCE
(Passenger Cars — Adjusted for Downgrades)
Figure 31-3B

US Customary					
Design Speed (mph)	Decision Sight Distance for Avoidance Maneuver (ft)				
	A	B	C	D	E
30	220	490	450	535	620
35	275	590	525	625	720
40	330	690	600	715	825
45	395	800	675	800	930
50	465	910	750	890	1030
55	535	1030	865	980	1135
60	610	1150	990	1125	1280
65	695	1275	1050	1220	1365
70	780	1410	1105	1275	1445
Metric					
Design Speed (km/h)	Decision Sight Distance for Avoidance Maneuver (m)				
	A	B	C	D	E
50	70	155	145	170	195
60	95	195	170	205	235
70	115	235	200	235	275
80	140	280	230	270	315
90	170	325	270	315	360
100	200	370	315	355	400
110	235	420	330	380	430

Note:

Avoidance Maneuver A: Stop on rural road.
Avoidance Maneuver B: Stop on urban road.
Avoidance Maneuver C: Speed/path/direction change on rural road.
Avoidance Maneuver D: Speed/path/direction change on suburban road.
Avoidance Maneuver E: Speed/path/direction change on urban road.

DECISION SIGHT DISTANCE

Figure 31-3C

- Avoidance Maneuver A: Stop on rural road.
- Avoidance Maneuver B: Stop on urban road.
- Avoidance Maneuver C: Speed/path/direction change on rural road.
- Avoidance Maneuver D: Speed/path/direction change on suburban road.
- Avoidance Maneuver E: Speed/path/direction change on urban road.

31-3.02(b) Applications

In general, the designer should consider using decision sight distance at any relatively complex location where the driver perception/reaction time may exceed 2.5 seconds. Example locations where decision sight distance may be appropriate include:

- freeway exit/entrance gores;
- freeway lane drops;
- freeway left-side entrances or exits;
- intersections near a horizontal curve;
- highway/railroad grade crossings;
- approaches to detours and lane closures;
- along high-speed, high-volume urban arterials with considerable roadside friction; or
- isolated traffic signals on high-speed rural highways.

As with SSD, the driver height of eye is 3.5 ft (1080 mm) and the height of object is typically 2 ft (600 mm). However, candidate sites for decision sight distance may also be candidate sites for assuming that the “object” is the pavement surface (e.g., freeway exit gores). Therefore, the designer may assume a 0.0 in (0.0 mm) height of object for application at some sites.

31-3.03 <u>Passing Sight Distance</u>

Passing sight distance only applies to two-lane, two-way highways. Therefore, its theoretical derivation and application are discussed in Chapter 47.

31-3.04 <u>Intersection Sight Distance</u>

Intersection sight distance applies to the determination of the sight triangle in the corners of at-grade intersections. Therefore, its theoretical derivation and application are discussed in Chapter 36.

31-4 TRAFFIC VOLUME CONTROLS

31-4.01 Definitions

1. Annual Average Daily Traffic (AADT). The total yearly volume in both directions of travel divided by the number of days in a year.

2. Average Daily Traffic (ADT). The calculation of average traffic volumes in both directions of travel in a time period greater than one day and less than one year and divided by the number of days in that time period. Although not precisely correct, ADT is often used interchangeably with AADT. The use of an ADT could produce a bias because of seasonal peaks and, therefore, the user should be aware of this.

3. Capacity. The maximum number of vehicles which can reasonably be expected to traverse a point or uniform section of a road during a given time period under prevailing roadway, traffic, and traffic control conditions. The time period most often used for analysis is 15 minutes. "Capacity" corresponds to Level of Service E.

4. Delay. The primary performance measure on interrupted flow facilities, especially at signalized intersections. For this element, average stopped-time delay is measured, which is expressed in seconds per vehicle.

5. Density. The number of vehicles occupying a given length of lane, averaged over time. It is usually expressed as vehicles per mile (kilometer) per lane.

6. Design Hourly Volume (DHV). The one-hour volume in both directions of travel in the design year selected for determining the dimensions and configuration of the highway design elements. For capacity analyses, the DHV is typically converted to an hourly flow rate based on the maximum 15-minute flow rate during the DHV.

7. Service Flow Rate. The maximum hourly vehicular volume that can pass through a highway element under prevailing roadway traffic and control conditions while maintaining a designated level of service.

8. Directional Design Hourly Volume (DDHV). The peak one-hour volume in one direction of travel during the DHV.

9. Directional Distribution (D). The division, by percent, of the traffic in each direction of travel, which is usually provided for the DHV. In some cases, D may be provided for the ADT and/or AADT.

10. Level of Service (LOS). A qualitative concept that has been developed to characterize acceptable degrees of congestion as perceived by motorists. In the *Highway Capacity Manual*, the qualitative descriptions of each level of service (A to F) have been

converted into quantitative measures for the capacity analysis for each highway element, including:

- freeway mainline;
- freeway mainline/ramp junctions;
- freeway weaving areas;
- interchange ramps;
- two-lane, two-way rural highways;
- multilane rural highways;
- signalized intersections;
- unsignalized intersections; and
- urban and suburban arterials.

11. Peak-Hour Factor (PHF). A ratio of the volume occurring during the peak hour to the maximum rate of flow during a given time period within the peak hour (typically 15 minutes).

12. Truck Factor (T). A factor that reflects the percentage of heavy vehicles (trucks, buses, and recreational vehicles) in the traffic stream during the DHV, ADT, and/or AADT. For geometric design and capacity analyses, trucks are defined as vehicles with six or more tires. Data on trucks are compiled by the districts and reported by the Office of Planning and Programming, Planning Services Section.

13. Rate of Flow. The equivalent hourly rate at which vehicles pass over a given point or section of a lane or roadway during a given time interval less than one hour (typically 15 minutes).

14. K. The ratio of DHV to AADT. K will vary based on the hour selected for design and the characteristics of the specific highway facility.

15. AM/PM Peak Volumes. The one-hour volumes for each movement at an intersection or interchange in the morning and evening. Both AM/PM peak volumes should be used for intersection and interchange analyses in suburban and urban areas where traffic volumes are high.

31-4.02 Design Year Selection

31-4.02(a) Roadway Design

The geometric design of a highway should be developed to accommodate expected traffic volumes during the life of the facility assuming reasonable maintenance. This involves projecting the traffic volumes to a selected future year. Recommended design years are presented in Figure 31-4A. The design year is measured from the expected construction completion date. Projected traffic volumes on State highways are provided by each district or from regional transportation studies with support from the Planning Services Section of the Office of Planning and Programming (OPP).

PROJECT SCOPE OF WORK	TYPICAL
New Construction/Reconstruction	20 Years
3R Freeway Projects	Current*
3R Non-Freeway Projects	Current*

* *In general, current traffic volumes may be used. However, if a 3R project will introduce a new geometric design element (e.g., relocation of a horizontal curve), the element should be designed based on reconstruction policies.*

RECOMMENDED DESIGN YEAR SELECTION
(Traffic Volumes for Road Design)

Figure 31-4A

31-4.02(b) Other Highway Elements

The following presents the recommended criteria for selection of a design year for highway elements other than road design:

1. Bridges. The structural life of a bridge may be 75 years or more. For new bridges, bridge replacement, and bridge reconstruction, the clear roadway width of the bridge will be based on the 20-year traffic volume projection beyond the construction completion date. In addition, the designer may, on selected projects, evaluate if the bridge design will reasonably accommodate structural expansion to meet the clear roadway width across the bridge based on a traffic volume projection beyond 20 years.

   For bridges within the limits of 3R projects, see Chapters 49 and 50.

2. Underpasses. The design year used for the geometric design of underpasses will be determined on a case-by-case basis.

3. Right-of-Way/Grading. The designer may consider potential right-of-way needs for the anticipated long-term corridor growth for a year considerably beyond that used for roadway design, especially in large metropolitan areas. No specific design year is recommended for use. For example, when selecting an initial median width on a divided highway, the designer may evaluate the potential need for future expansion of the facility

to add through travel lanes. Other examples include potential future interchanges, potential reconstruction of a two-lane, two-way facility to a multilane highway, and the use of flatter side slopes to provide more future options.

4. Drainage Design. Drainage appurtenances are designed to accommodate a flow rate based on a specific design year (or frequency of occurrence). The selected design year or frequency will be based on the functional class of the facility, the ADT, and the specific drainage appurtenance (e.g., culvert). The IDOT *Drainage Manual* presents the Department's criteria for selecting the frequency of occurrence. The design life of new drainage structures is typically 50 years.

5. Pavement Design. The pavement structure is designed to withstand the vehicular loads during the design analysis period without falling below a selected pavement serviceability rating. Chapter 54 presents the Department's criteria for selecting a design year for pavements.

6. Environmental Analyses. Some environmental analyses require the selection of a future year for design (e.g., noise analyses). BDE determines the specific criteria for environmental analyses.

31-4.03 Design Hourly Volume Selection

For most geometric design elements which are determined by traffic volumes, the peaking characteristics are most significant. The highway facility should be able to accommodate the design hourly volume (adjusted for the peak-hour factor) at the selected level of service. This design hourly volume (DHV) will affect many design elements including the number of through travel lanes, lane and shoulder widths, and intersection geometrics. The designer should also analyze the proposed design using the a.m. and p.m. DHVs separately. This could have an impact on the geometric design of the highway.

Traditionally, the 30th highest hourly volume in the selected design year has been used to determine the DHV for design purposes. This is still considered appropriate for rural facilities. However, at the discretion of the district, for urban facilities it may be more appropriate to base the DHV on the 10th to 20th highest hourly volume in the selected design year. See the *Highway Capacity Manual* for more detailed discussion on selecting the DHV. Because the design of the project is significantly dependent upon the projected design hourly volumes, these projections must be carefully examined before using for design purposes.

31-4.04 Level of Service

Level of service (LOS) describes a qualitative measure of operational conditions within a traffic stream as perceived by motorists. A designated LOS is described in terms of average travel speed, density, traffic interruptions, comfort, convenience, and safety.

Because drivers will accept different driving operational conditions, including lower travel speeds on different facilities, it is not practical to establish one level of service for application to every type of highway. Therefore, several levels have been established for the various classes and types of highways. The values of speed and design hourly volume used in each case to identify a level of service are the lowest acceptable speed and highest obtainable volume for that specific level.

Part V, Design of Highway Types, presents LOS criteria for each highway type.

31-4.05 Capacity Analyses

31-4.05(a) Objective

The highway mainline, intersection, or interchange should be designed to accommodate the selected design hourly volume (DHV) at the selected level of service (LOS). This may involve adjusting the various highway factors which affect capacity until a design is determined that will accommodate the DHV. The detailed calculations, factors, and methodologies are presented in the *Highway Capacity Manual* (HCM).

The designer should note that, in reality, the service flow rate of the facility is calculated. Capacity assumes a LOS E, and the service flow rate is the maximum volume of traffic that a proposed highway of given geometrics is able to serve without the degree of congestion falling below a selected LOS. This is almost always higher than LOS E.

The HCM has established measures of effectiveness (MOE) for the level-of-service definition for each highway element on various types of highway facilities. These are presented in Figure 31-4B. For each MOE, the HCM will provide the analytical tools to calculate the numerical value. The designer should note that highway capacity MOEs may be segregated into two broad categories: (1) uninterrupted flow, or open highway conditions, and (2) interrupted flow, as at stop-controlled or signalized intersections. Uninterrupted flow occurs on highways where the influence of intersections and abutting property development is not significant, and the design volume of a facility can be determined by an hourly rate of flow.

TYPE OF FACILITY	MEASURE OF EFFECTIVENESS
Freeways	
Basic freeway segments	Density (passenger cars per mile (kilometer) per lane)
Weaving areas	Average travel speed (mph (km/hr))
Ramp junctions	Flow rates (passenger cars per hour)
Multilane Highways	Density (passenger cars per mile (kilometer) per lane)
Two-Lane, Two-Way Highways	Percent time delay (%) Average travel speed (mph (km/hr))
Signalized Intersections	Average individual stopped delay (seconds/vehicle)
Unsignalized Intersections	Reserve capacity (passenger cars per hour)
Arterials	Average travel speed (mph (km/hr))

MEASURES OF EFFECTIVENESS FOR LEVEL OF SERVICE

Figure 31-4B

The following presents the simplified procedure for conducting a capacity analysis for the highway mainline:

1. Select the design year (Section 31-4.02).

2. Determine the DHV (Section 31-4.03).

3. Select the level of service (see Part V, Design of Highway Types).

4. Document the proposed highway geometric design (lane width, length of weaving section, number and width of approach lanes at intersections, etc.).

5. Using the HCM, analyze the capacity of the highway element for the proposed design:

    - determine the maximum flow rate under ideal conditions;

    - adjust the maximum flow rate for prevailing roadway, traffic, and traffic control conditions; and

- calculate the service flow rate for the selected level of service.

6. Compare the calculated service flow rate to the DHV. If the DHV is less than or equal to the service flow rate, the proposed design will meet the objectives of the capacity analysis. If the DHV exceeds the service flow rate, the proposed design may need further evaluation. The designer should either adjust the highway design or should adjust one of the capacity elements (e.g., the selected design year or the level-of-service goal).

31-4.05(b) Responsibility

For IDOT projects, the district Geometrics Engineer (or sometimes the project engineer) is responsible for performing all capacity analyses required by the project. The Policy and Procedures Section or the Project Development and Implementation Section is available as a resource to the district to assist in all capacity analyses.

For consultant-designed projects, the consultant is responsible for performing capacity analyses. Before submission to the Central Office, the district Geometrics Engineer reviews the results. Consultants must use only highway capacity software that is approved by BDE.

31-4.06 <u>Maximum Hourly Volume (MHV)</u>

For general design purposes, IDOT uses a volume threshold for the various highway classes designated as the maximum hourly volume (MHV). This denotes the maximum volume that can be accommodated at a selected level of service based on a typical set of operational assumptions for each facility. The peak-hour factors for levels of service B and C are 0.92 and 0.94 respectively. These values are based on the assumption of random flow and are generally higher than those obtained from field observations.

The geometric design tables in Part V, Design of Highway Types, present the MHV or service flow rate values for the applicable highway types. For arterial multilane highways, a maximum of 2100 passenger cars per hour per lane (pcphpl) is used as a base volume, which must be adjusted for design and analysis to reflect the prevailing roadway and traffic conditions. As discussed in the HCM, these adjustments include volume-to-capacity ratio, number of lanes, lane width, percent of heavy vehicles, driver population, and peak-hour factor. The equation for determining the maximum hourly volume is (2100) (v/c) (N) (f_w) (f_{hv}) (f_p) (phf). Additional adjustments for specific grades may also be necessary.

The MHVs shown in the tables in Part V for multilane highways are one-way volumes derived from the above equation using appropriate HCM values and based on the following truck percentages for f_{hv}: Freeways – 16% and Expressways – 8%. However, a PHF = 1.0 has been assumed, and the values in the Part V tables for multilane highways must be multiplied by the actual PHF. For two-lane arterials, truck percentages are 7% and 6%, respectively. (For calculation of two-way DHV on multilane highways, a 60%/40% directional distribution should be assumed in the absence of better data.)

The MHV for two-lane rural arterials under ideal conditions is 2800 pcph total in both directions. This volume must be adjusted to reflect the prevailing roadway and traffic conditions. These adjustments include volume-to-capacity ratio, directional distribution, lane width, heavy vehicles, and peak-hour factor for an assumed level of service. The equation for determining the MHV for two-lane highways is: (2800) (v/c) (f_d) (f_w) (f_{hv}) (phf). Additional adjustments for specific grades may also be necessary. The volumes shown in Part V for rural two-lane highways are based on 100% passing sight distance, a 60%/40% directional distribution and other appropriate values from the HCM. On a project length basis, as much passing sight distance as practical should be provided with approximately 60% available as a minimum for level terrain and approximately 40% as a minimum for rolling terrain. The actual allowable MHV must be determined for each project based on the actual percentage of passing sight distance provided and any adjustment factors other than those normally used.

31-5 NON-HIGHWAY DESIGN CONTROLS

The characteristics of drivers and vehicles significantly influence the selected design criteria. When the driver and vehicle are properly accommodated, the safety and serviceability of the highway system are enhanced. When they are not accommodated, crashes and inefficient operation may result.

31-5.01 Driver

31-5.01(a) Typical Driver

The appropriate considerations for drivers are already built into the applicable geometric design values (stopping sight distance, horizontal curvature, superelevation, roadway widths, etc.). However, a brief discussion of the "typical" driver is warranted.

Drivers vary widely in their operating skills, experience, intelligence, and physical condition. The highway should be as forgiving as practical to minimize the adverse effects of driver errors. The following discusses certain principles and driver traits that should be incorporated into the roadway design:

1. Information Processing. Drivers are limited in how quickly they can gather information, make a decision, and take action. They must process information related to lane placement, speed, traffic control devices, highway alignment, roadside conflicts, and weather. If the amount, complexity, or clarity of the information is inappropriate or excessive, driver error leading to an accident can result.

2. Primacy. Certain driving functions are more important than others. In order of importance they are:

   - Control — activities related to the physical control of the vehicle via the steering wheel, brake, or accelerator.

   - Guidance — activities related to selecting a safe speed and vehicular path on the highway.

   - Navigation — activities related to planning and executing a trip from point of origin to destination.

   The roadway designer must be aware of the relative importance of these activities and ensure that the more important highway information is properly conveyed to the driver. This could result in the decision to remove or relocate lower priority information, if it is likely to interfere with the higher priority information.

3. Expectancy. Drivers are conditioned through experience and training to expect and anticipate what lies ahead on the highway. If this driver expectancy is violated, it will increase the time needed by the driver to assess the situation and make the correct decision. These violations should be avoided. Where they are unavoidable, the designer should allow for increased warning time.

4. Speed. Speed must be considered when accommodating the driver. Higher speeds reduce the visual field and restrict peripheral vision.

A User's Guide to Positive Guidance (FHWA) contains more detailed information related to driver characteristics and highway design accommodation for the driver.

31-5.01(b) Elderly Driver

In general, the median age of drivers in the United States is increasing and, specifically, the age bracket of over 60 years is the fastest growing segment of the driver population. This reality greatly emphasizes the criticality of the relationship between the driver and the highway environment. Although the opinions are not unanimous, there is general agreement that advancing age has a deleterious effect on an individual's perceptual, mental, and motor skills — critical factors in vehicular operation.

The research community has conducted several studies of the elderly driver, including:

- "Older Driver Study of Traffic Control Devices in Illinois," Illinois Department of Transportation, 1991;
- "Highway Design and Traffic Operation Needs of Older Drivers," University of Illinois at Urbana - Champaign, January 1994;
- "Strategies for Improving the Safety of Elderly Drivers," University of Nebraska/Midwest Transportation Center, 1991; and
- "Highway Design Handbook for Older Drivers and Pedestrians," FHWA, 2001.

These four studies were primarily focused on the relationship between the elderly driver and traffic control devices where, arguably, a greater opportunity exists for cost-effective countermeasures than for roadway design. However, it is important for the road designer to be aware of the needs of the elderly driver and, where desirable, factor these needs into the roadway design. The following summarizes the more important observations from these studies:

1. Elderly Driving Characteristics. When compared to younger drivers, the elderly driver often exhibits the following operational deficiencies:

   - slower information processing;
   - slower reaction times;
   - slower decision making;
   - visual deterioration;
   - hearing deterioration;
   - decline in ability to judge time, speed, and depth perception;
   - limitations on physical mobility; and
   - side effects from prescription drugs.

2. Crash Frequency. Predictably, elderly drivers are involved in a disproportionate number of crashes where there is a higher than average demand imposed on driving skills. The driving maneuvers that most often precipitate higher crash frequencies among older drivers include:

   - left turns across traffic,
   - merging with high-speed traffic,
   - changing lanes on congested streets,
   - crossing high-volume intersections,
   - need to stop quickly for queued traffic,
   - backing maneuvers, and
   - parking.

3. Countermeasures. The studies identified several countermeasures to alleviate the potential problems of the elderly driver. These included:

   - increasing driver education,
   - increasing vehicular clearance times at signalized intersections,
   - increasing pedestrian phase times,
   - providing wider and brighter pavement markings,
   - providing larger and brighter signs,
   - reducing sign clutter,
   - providing more redundant information (such as advance guide signs),
   - installing grade separations,
   - revising warrants for traffic signals to increase their usage,
   - enforcing speed limits,
   - widening intersections,
   - increasing use of protected left-turn phases, and
   - increasing sight distance.

Most of the proposed countermeasures are related to traffic control devices. Perhaps the most practical measure related to road design is increasing sight distance. From an implementation perspective, this recommendation may be related to the warrants for the use of decision sight distance, as discussed in Section 31-3. The gradual aging of the driver population suggests that an increased use of decision sight distance may produce a commensurate reduction in the crash frequency for elderly drivers. These findings suggest that, where decision sight distance cannot physically be provided, an increased use of advance warning signs may be appropriate.

31-5.02 Vehicle

The physical and operational characteristics of vehicles using the highway are important controls in roadway design. Design criteria may vary according to the type of vehicle and the volume of each type of vehicle in the traffic stream.

Vehicular characteristics that impact design include:

1. Size. Vehicular sizes determine lane and shoulder widths, vertical clearances and, indirectly, highway capacity calculations.

2. Offtracking. The design of intersection turning radii, traveled way widening for horizontal curves, and pavement widths for interchange ramps are usually controlled by the largest design vehicle likely to use the facility with some frequency.

3. Storage Requirements. Turn bay storage lengths, bus turnouts, and parking lot layouts are determined by the number and types of vehicles to be accommodated.

4. Sight Distance. Eye height and braking distances vary for passenger cars and trucks, which can impact sight distance considerations.

5. Acceleration and Deceleration. Acceleration and deceleration rates often govern the dimensioning of such design features as speed-change lanes at intersections and interchange ramps and climbing lanes.

6. Vehicular Stability. Certain vehicles with high centers of gravity may be prone to skidding or overturning, affecting design speed selection and superelevation design elements.

Figures 31-5A and 31-5B present vehicular dimensions and minimum turning radii for typical design vehicles. Figures 31-5C and 31-5D present two combination trucks to illustrate the application of the basic dimensions.

The selection of appropriate design vehicles for intersections and interchanges is discussed in Chapters 36 and 37, respectively.

Design Vehicle Type	Symbol	Dimensions (feet)										
		Overall			Overhang		Wheelbases					Typical Kingpin to Center of Rear Axle
		Height	Width	Length	Front	Rear	WB_1	WB_2	S	T	WB_3	
Passenger car	P	4.25	7	19	3	5	11	–	–	–	–	–
Single unit truck	SU	11-13.5	8.0	30	4	6	20	–	–	–	–	–
City transit bus	CITY-BUS	10.5	8.5	40	7	8	25	–	–	–	–	–
Articulated bus	A-BUS	11.0	8.5	60	8.6	10	22.0	19.4	6.2[a]	13.2[a]	–	–
School bus (84 passenger)	S-BUS	10.5	8.0	40	7	13	20	–	–	–	–	–
Combination trucks:												
Intermediate Semitrailer	WB-40	13.5	8.0	45.5	3	2.5[a]	12.5	27.5	–	–	–	27.5
Large Semitrailer	WB-50	13.5	8.5	55	3	2[a]	14.6	35.4	–	–	–	37.5
Large Semitrailer*	WB-55	13.5	8.5	66	3.5	7.5	14.6	40.4	–	–	–	42.5
Semitrailer - Full Trailer (“Double Bottom”)	WB-67D	13.5	8.5	73.3	2.33	3	11.0	23.0	3.0[b]	7.0[b]	23.0	23.0
Interstate Semitrailer*	WB-65	13.5	8.5	73.5	4	4.5-2.5[a]	21.6	43.4-45.4	-	–	–	45.5-47.5
Recreational vehicles:												
Motor home	MH	12	8	30	4	6	20	–	–	–	–	–
Car and camper trailer	P/T	10	8	48.7	3	10	11	–	5	19	–	–
Car and boat trailer	P/B	10	8	42	3	8	11	–	5	15	–	–
Motor home and boat trailer	MH/B	12	8	53	4	8	20	–	6	15	–	–

** On semitrailers longer than 48 ft, the maximum distance between the kingpin and the rear axle shall not exceed 45.5 ft.*

a = Combined dimension of 19.4 ft is typical.
b = Combined dimension of 10.0 ft is typical.

WB_1, WB_2, WB_3 are effective vehicle wheelbases, starting at the front and moving towards the back of the vehicle.
S is the distance from the rear effective axle of a vehicle to the hitch point or, for A-BUS, the distance from second axle to articulating section.
T is the distance from the hitch point of a vehicle to the lead effective axle or axle set of the following unit or, for A-BUS, the distance from articulating section to rear axle.

TYPICAL DESIGN VEHICLE DIMENSIONS
(US Customary)

Figure 31-5A

Design Vehicle Type	Symbol	Dimensions (meters)										
		Overall			Overhang		Wheelbases					Typical Kingpin to Center of Rear Axle
		Height	Width	Length	Front	Rear	WB_1	WB_2	S	T	WB_3	
Passenger car	P	1.3	2.1	5.8	0.9	1.5	3.4	–	–	–	–	–
Single unit truck	SU	3.4-4.1	2.4	9.2	1.2	1.8	6.1	–	–	–	–	–
City transit bus	CITY-BUS	3.2	2.6	12.2	2.1	2.4	7.6	–	–	–	–	–
Articulated bus	A-BUS	3.4	2.6	18.3	2.6	3.1	6.7	5.9	1.9[a]	4.0[a]	–	–
School bus (84 passenger)	S-BUS	3.2	2.4	12.2	2.1	4.0	6.1	–	–	–	–	–
Combination trucks:												
Intermediate Semitrailer	WB-12	4.1	2.4	13.9	0.9	0.8[a]	3.8	8.4	–	–	–	8.4
Large Semitrailer	WB-15	4.1	2.6	16.8	0.9	0.6[a]	4.5	10.8	–	–	–	11.4
Large Semitrailer*	WB-17	4.1	2.6	20.19	1.13	2.29	4.45	12.32	–	–	–	13.0
Semitrailer - Full Trailer (“Double Bottom”)	WB-20D	4.1	2.6	22.4	0.7	0.9	3.4	7.0	0.9[b]	2.1[b]	7.0	7.0
Interstate Semitrailer*	WB-20	4.1	2.6	22.4	1.2	1.4-0.8[a]	6.6	13.2-13.8	–	–	–	13.9–14.5
Recreational vehicles:												
Motor home	MH	3.7	2.4	9.2	1.2	1.8	6.1	–	–	–	–	–
Car and camper trailer	P/T	3.1	2.4	14.0	0.9	3.1	3.4	–	1.5	5.8	–	–
Car and boat trailer	P/B	3.1	2.4	12.8	0.9	2.4	3.4	–	1.5	4.6	–	–
Motor home and boat trailer	MH/B	3.7	2.4	16.2	1.2	2.4	6.1	–	1.8	4.6	–	–

** On semitrailers longer than 14.63 m, the maximum distance between the kingpin and the rear axle shall not exceed 13.87 m.*

a = Combined dimension of 5.91 m is typical.
b = Combined dimension of 3.25 m is typical.

WB_1, WB_2, WB_3 are effective vehicle wheelbases, starting at the front and moving towards the back of the vehicle.
S is the distance from the rear effective axle of a vehicle to the hitch point or, for A-BUS, the distance from second axle to articulating section.
T is the distance from the hitch point of a vehicle to the lead effective axle or axle set of the following unit or, for A-BUS, the distance from articulating section to rear axle.

TYPICAL DESIGN VEHICLE DIMENSIONS
(Metric)

Figure 31-5A

Design Vehicle Type	Passenger Car	Single Unit Truck	City Transit Bus	Articulated Bus	School Bus	Intermediate Semi-Trailer	Intermediate Semi-Trailer	Large Semi-Trailer	Semi-Trailer/Full Trailer (Double Bottom)	Inter-State Semi-Trailer	Motor Home	Passenger Car with Camper	Passenger Car with Boat Trailer	Motor Home and Boat Trailer
Symbol	P	SU	CITY-BUS	A-BUS	S-BUS	WB-40	WB-50	WB-55	WB67D	WB-65[1]	MH	P/T	P/B	MH/B
Minimum design [2] turning radius (ft)	24	42	42.0	39.8	39.4	40	45	45	45	45	40	33	24	50
Centerline [3] turning radius (CTR) (ft)	21	38	37.8	35.5	35.4	36	41	41	41	41	36	30	21	46
Minimum inside radius (ft)	14.4	28.3	24.5	21.3	25.4	19.3	17.0	18.4	19.3	4.4	25.9	17.4	8.0	35.1

Notes:

1. *Design vehicle with 53 ft trailer as adopted in 1982 STAA (Surface Transportation Assistance Act).*
2. *Minimum design turning radius is measured to the outside of the left front wheel of vehicle as determined by the vehicle steering mechanism.*
3. *CTR = the turning radius assumed by the designer when investigating possible turning paths and is set at the centerline of the front axle of a vehicle. See Chapter 36.*

MINIMUM TURNING RADII OF TYPICAL DESIGN VEHICLES
(US Customary)

Figure 31-5B

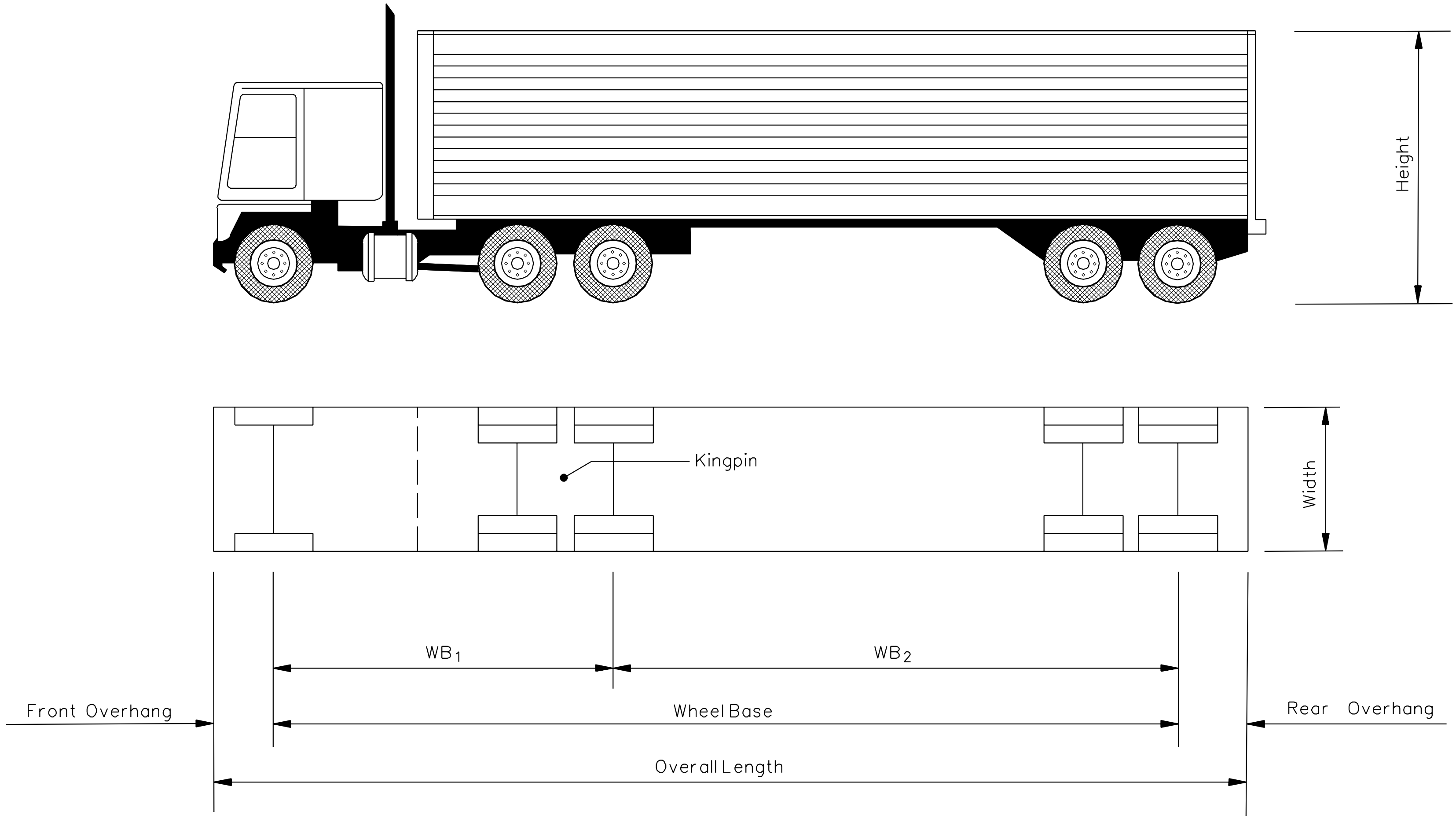
Design Vehicle Type	Passenger Car	Single Unit Truck	City Transit Bus	Articulated Bus	School Bus	Intermediate Semi- Trailer	Intermediate Semi-Trailer	Large Semi-Trailer	Semi-Trailer/Full Trailer (Double Bottom)	Inter-State Semi-Trailer	Motor Home	Passenger Car with Camper	Passenger Car with Boat Trailer	Motor Home and Boat Trailer
Symbol	P	SU	CITY-BUS	A-BUS	S-BUS	WB-12	WB-15	WB-17	WB-20D	WB-20 (1)	MH	P/T	P/B	MH/B
Minimum design (2) turning radius (m)	7.3	12.8	12.8	12.1	12.0	12.2	13.7	13.7	13.7	13.7	12.2	10.1	7.3	15.2
Centerline (3) turning radius (CTR) (m)	6.4	11.6	11.5	10.8	10.8	11.0	12.5	12.5	12.5	12.5	11.0	9.1	6.4	14.0
Minimum inside radius (m)	4.4	8.6	7.5	6.5	7.7	5.9	5.2	5.6	5.9	1.3	7.9	5.3	2.8	10.7

Notes:

1. *Design vehicle with 16.16 m trailer as adopted in 1982 STAA (Surface Transportation Assistance Act).*
2. *Minimum design turning radius is measured to the outside of the left front wheel of vehicle as determined by the vehicle steering mechanism.*
3. *CTR = the turning radius assumed by the designer when investigating possible turning paths and is set at the centerline of the front axle of a vehicle. See Chapter 36.*

MINIMUM TURNING RADII OF TYPICAL DESIGN VEHICLES
(Metric)

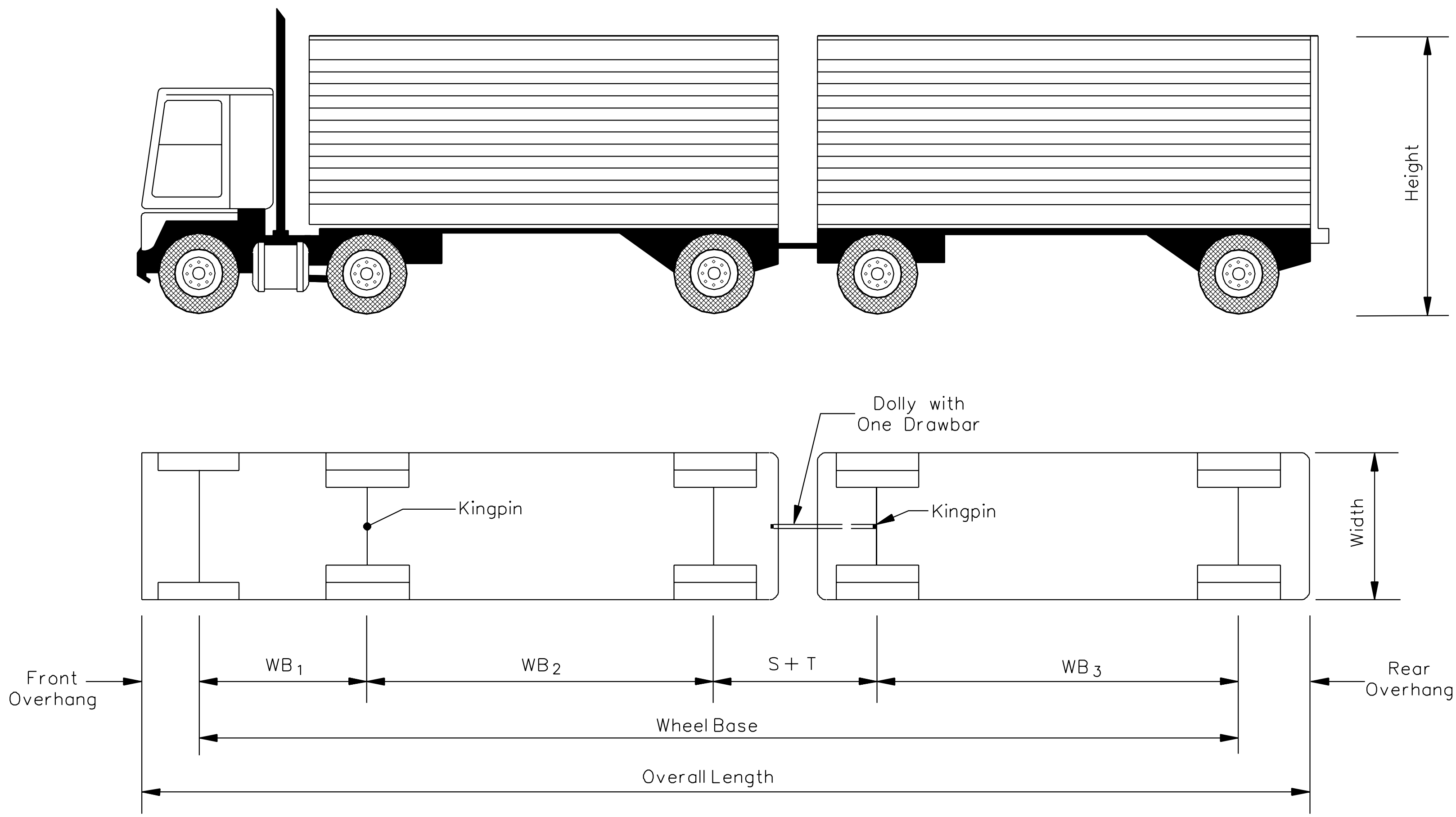
Figure 31-5B



Note: For the legal dimensions of trucks allowed on Illinois highways, refer to information from the Central Bureau of Operations, Maintenance Operations Section.

BASIC DIMENSIONS OF TRACTOR-SEMITRAILER VEHICLE

Figure 31-5C



Note: For the legal dimensions of trucks allowed on Illinois highways, refer to information from the Central Bureau of Operations, Maintenance Operations Section.

BASIC DIMENSIONS OF TRACTOR-SEMITRAILER/TRAILER VEHICLE
(Double Bottom)

Figure 31-5D

31-5.03 Pedestrians

The pedestrian must be considered as an integral part of the highway environment, especially in urban areas. Except on fully access-controlled facilities, pedestrians are legally allowed to use the highway right-of-way consistent with the restrictions placed on pedestrian use. Therefore, the roadway design should provide for the safe and efficient movement of pedestrians, within practical limits, without compromising the accommodation of the vehicles using the highway facility.

The *BDE Manual* presents many specific design criteria for the accommodation of pedestrians as follows:

- Chapter 17 discusses pedestrian safety.
- Chapter 58 discusses accessibility criteria.
- Chapter 48 discusses sidewalks.
- Chapter 36 discusses pedestrian accommodation at intersections.
- Chapter 56 discusses pedestrian accommodation with traffic signals.

31-5.04 Bicyclists

Similar to pedestrians, bicyclists are an important element of the highway environment. Chapter 17 discusses the detailed design criteria for bicycle accommodation.

31-6 PROJECT SCOPE OF WORK

The project scope of work will reflect the basic intent of the highway project and will determine the overall level of highway improvement. This decision, in combination with the highway functional classification (see Chapter 43), will determine which criteria in the *Manual* apply to the geometric design of the project. The following provides general definitions for the project scopes of work, and it references the applicable chapters in Part V, Design of Highway Types, for the design criteria based on the project scope of work.

31-6.01 New Construction

Generally, new construction is defined as horizontal and vertical alignment on new location. The development is based on at least a 20-year design period. Typically, the project will have a significant length and will connect major termini. Where an existing two-lane, two-way facility becomes a multilane facility with a rural-type median, the new median and proposed roadway are considered new construction. In addition, new construction also includes any intersection or interchange that falls within the project limits of a new highway mainline or is relocated to a new point of intersection. Freeways, expressways, and bypasses are the typical new construction projects. Chapters 44 through 48 present IDOT criteria for new construction.

31-6.02 Reconstruction

Reconstruction of an existing highway will typically include the addition of travel lanes and/or reconstruction of the existing horizontal and vertical alignment, widening of the roadway, and flattening side slopes, but the highway will remain essentially within the existing highway corridor. These projects will usually require some right-of-way acquisitions. The primary reasons for reconstructing an existing highway are because the facility cannot accommodate its current or future traffic demands, because the existing alignment or cross section is deficient, and/or because the service life of the pavement has been exceeded. In addition, any intersection that falls within the limits of a reconstruction project will be reconstructed as needed.

Because of the significant level of work for reconstruction, the design of the project generally will be determined by the criteria for new construction based on a 20-year design period. However, some existing cross section elements may be allowed to remain in place. Chapters 44 through 48 will apply to reconstruction projects.

31-6.03 3R Projects (Non-Freeways)

3R projects (rehabilitation, restoration, and/or resurfacing) on non-freeways are primarily intended to extend the service life of the existing facility and to enhance highway safety. In addition, 3R projects should make cost-effective improvements to the existing geometrics, where practical. 3R work on the mainline or at an intersection is typically work within the existing alignment. However, right-of-way acquisition is sometimes justified for flattening slopes, changes in horizontal alignment, changes in vertical profile, and safety enhancements.

The overall objective of a 3R non-freeway project is to perform work necessary to return the highway to a condition of acceptable structural and/or functional adequacy. 3R projects may include any number of the following types of improvements:

- providing pavement resurfacing, rehabilitation, and/or short sections of pavement reconstruction;
- providing lane and/or shoulder widening (without adding through lanes);
- adding a two-way, left-turn lane (TWLTL);
- providing intersection improvements (e.g., adding turn lanes, flattening turning radii, channelization, corner sight distance improvements);
- flattening a horizontal or vertical curve;
- adding auxiliary lanes (e.g., truck-climbing lane);
- converting an existing uncurbed urban street into a curbed street;
- widening and/or resurfacing parking lanes;
- upgrading at-grade railroad crossings;
- rehabilitating and/or widening existing bridges;
- upgrading guardrail and other roadside safety appurtenances to meet current criteria;
- adjusting the roadside clear zone;
- flattening side slopes;
- providing drainage improvements, including pump stations; and/or
- implementing improvements to meet the Department's accessibility criteria (e.g., sidewalks and sidewalk curb ramps).

Any of the above may also be an element of work for a reconstruction project. Chapter 49 presents IDOT criteria for the design of 3R non-freeway projects.

31-6.04 3R Projects (Freeways)

3R projects (resurfacing, restoration, and/or rehabilitation) on existing freeways are primarily intended to extend the service life of the existing facility and to enhance highway safety. In addition, these projects should make cost-effective improvements to the existing geometrics, where practical. 3R freeway projects may include any number of the following types of improvements:

- providing pavement resurfacing, rehabilitation, and/or short sections of pavement reconstruction;
- realigning or widening an existing ramp or modifying an existing interchange;
- lengthening existing acceleration or deceleration lanes at freeway entrances and exits;
- flattening a horizontal or vertical curve;
- adding auxiliary lanes (e.g., a truck-climbing lane);
- rehabilitating and/or widening existing bridges;
- upgrading guardrail and other roadside safety appurtenances to meet current criteria;
- adjusting the roadside clear zone;
- flattening side slopes; and/or
- providing drainage improvements, including pump stations.

Chapter 50 presents IDOT criteria for the design of 3R freeway projects.

31-7 FHWA OVERSIGHT AND INVOLVEMENT

31-7.01 Background

Prior to the passage of the Intermodal Surface Transportation Efficiency Act (ISTEA) in 1991, the Federal-Aid Highway Program had focused on the construction and improvement of four Federal-aid Systems – Interstate, Primary, Secondary, and Urban. ISTEA provided authorizations for highways, highway safety, and mass transportation for the next six years. This legislation contained major changes concerning the highway funding program. ISTEA provided for three Federal funding program categories:

- Interstate,
- National Highway System (NHS), and
- Surface Transportation Program (STP).

See Section 43-3 for a discussion on the Federal-aid funding categories.

ISTEA necessitated changes in the working relationship between the Department and FHWA, especially for the type and extent of oversight on Federal-aid projects.

TEA-21, signed in 1998, maintains the Federal funding categories of ISTEA, but this Act precipitated further changes in Federal oversight actions on State highway projects.

31-7.02 Project Oversight Agreement

31-7.02(a) Introduction

Pursuant to TEA-21, the Division of Highways and the Illinois FHWA Division signed an oversight agreement on September 28, 1999. The terms of the Project Oversight Agreement are summarized in this Section.

The Project Oversight Agreement was developed to meet the requirements of Section 106(c)(3) of Title 23 United States Code (USC) – Agreement. – "*The Secretary and the State shall enter into an agreement relating to the extent to which the State assumes the responsibilities of the Secretary under this subsection.*" The Illinois Department of Transportation (IDOT) is taking advantage of the opportunities afforded State highway agencies under Section 106. These opportunities relate to the Federal Highway Administration's (FHWA) approval and oversight on projects on and off the National Highway System.

31-7.02(b) Oversight

The following applies:

1. Interstate System. On New Construction/Reconstruction (4R) projects (see Figure 31-7A) with construction costs greater than $1 million, all project activities shall be developed with full FHWA oversight and approval as shown in Figure 31-7B. Upon agreement by the FHWA Transportation Engineer and the IDOT Design and Environment Field Engineer, large or complex rehabilitation projects will also be considered for full FHWA oversight.

2. All other Projects with Federal Funding Participation. IDOT will assume all responsibilities in accordance with Section 106 of Title 23 USC. This applies to all design activities, PS&E approvals, concurrence in awards, and construction and maintenance activities. This precludes the need for any FHWA approval or concurrence, except those actions that require FHWA approval outside of Title 23 USC (e.g., National Environmental Policy Act (NEPA), Title VI of the Civil Rights Act, Fair Housing Act, and the Uniform Relocation Assistance and Land Acquisitions Policies Act).

<table>
<tr><th rowspan="2"></th><th colspan="3">Project Category</th></tr>
<tr><th>Preventive Maintenance</th><th>3R</th><th>4R</th></tr>
<tr><td rowspan="2">Pavement</td><td>Thin Overlays</td><td rowspan="2">Structural Overlays</td><td rowspan="2">Pavement Replacement</td></tr>
<tr><td>Pavement Patching</td></tr>
<tr><td rowspan="2">Bridge</td><td>Deck Patching</td><td>Deck Overlay/Replacement</td><td>Superstructure Replacement</td></tr>
<tr><td>Substructure Repair Superstructure Painting</td><td>Rail Replacement (incl. minor deck widening)</td><td>Substructure widening that adds a lane width or more</td></tr>
<tr><td rowspan="2">Traffic Operations</td><td rowspan="2">Pavement Marking & Signing</td><td>Traffic Operation and Safety Projects</td><td>Interchange Reconstruction</td></tr>
<tr><td>Add Auxiliary Lanes</td><td>Add Through Lanes</td></tr>
</table>

Note: For projects with a combination of 3R and 4R activities, the category with the majority (i.e., greatest cost) of work will govern.

PROJECT CATEGORIES

Figure 31-7A

Oversight	Interstate				NHS (non-I)	City of Chicago (NHS & non-NHS)	non-NHS
	Full Oversight New or 4R > $1 million (3R by Agreement)	Exempt 3R	Preventive Maintenance	State-funded Projects	Exempt		Exempt
Governing Policy	FHWA Policy	FHWA Policy with Approval Actions Delegated to IDOT	FHWA Policy with Approval Actions Delegated to IDOT	State Policy FHWA Design Stds.	FHWA Policy with Approval Actions Delegated to IDOT		State Policy
FHWA ACTION							
Environmental Approval	Required			N/A	Required		
FHWA Design Approval	Required	IDOT	Not required for retention of existing conditions	IDOT	IDOT		State Policy and Procedures
Mainline Exceptions Other Exceptions	Formal Submittal At Coord. Mtg.	Formal Submittal At Coord. Mtg.		Formal submittal at Coord. Mtg.			
Safety Review and/or Pre-final Plan Review	Required (part of PS&E approval)	Determine at Coord. Mtg.	IDOT	N/A			
PS&E Approval	Required	IDOT	IDOT	N/A			
Authorization	Required			N/A			
Concurrence in Award	Required	IDOT	IDOT	N/A	IDOT		State Policy and Procedures
Change Order Approval	Required – Advance Approval is Required for Major Changes*	IDOT Except scope or termini changes and payment of premium pay or escalated prices	IDOT Except scope or termini changes and payment of premium pay or escalated prices	N/A	IDOT		
Claims Time Extension	Required Required				IDOT		
Materials Certification	Required	IDOT	IDOT	N/A	IDOT		IDOT Procedures
FHWA-45 & FHWA-47	Completion, approval, and transmittal to FHWA HQ-IDOT			N/A	IDOT		N/A
Project Inspection	Inspections-in-depth & included in PR/PE sampling	Significant projects included in PR/PE sampling	Significant projects included in PR/PE sampling	N/A	IDOT		
Final Inspection	IDOT			N/A	IDOT		

* *Major Change – as defined by the current Construction Memorandum XX-4, Contract Changes.*

FHWA PROJECT OVERSIGHT ACTIONS

Figure 31-7B

31-7.02(c) Control Documents

Control documents establish project development or project implementation procedures and are incorporated into project contract documents. Memorandums and Supplementals are considered control documents.

The following IDOT control documents will be adhered to in the development and administration of Federal-aid projects:

- *Bureau of Design and Environment Manual*,
- *Construction Manual*,
- *Standard Specifications for Road and Bridge Construction*,
- *Bureau of Operations Traffic Policies and Procedures Manual*,
- *Highway Standards*,
- *Bridge Manual*, and
- *Bureau of Local Roads and Streets Manual*.

The FHWA review and approval of changes to control documents is a program-level review activity. The FHWA review of control documents will be through participation on the Specification Committee and participation in the revision or rewriting of such documents. The application and implementation of procedures established in the control documents will be reviewed on a program-level as part of the joint FHWA/IDOT process review program.

31-7.02(d) Laws and Regulations

IDOT will follow all applicable Federal-aid laws and regulations, including the following, plus any other applicable laws and regulations:

- Title 23 U.S.C. – Highways,

- 23 CFR – Code of Federal Regulations Highways,

- 49 CFR Part 26 – Participation by Disadvantaged Business Enterprises in Department of Transportation Financial Assistance Programs,

- The Federal Managers' Financial Integrity Act of 1982,

- *A Policy on Geometric Design of Highways and Streets* – AASHTO (*Green Book*),

- *A Policy on Design Standards – Interstate System* – AASHTO,

- *Manual on Uniform Traffic Control Devices* (MUTCD), and

- *Highway Safety Design and Operations Guide 1997* – AASHTO (*Yellow Book*).

31-7.02(e) Obligation of Funds

IDOT will request obligation of funds and FHWA will respond to those requests using FHWA's electronic financial management program (FMIS).

IDOT will not submit requests for obligation of funds on any Federal-aid project until the National Environmental Policy Act (NEPA) approval process has been completed and the projects for which funds are being sought are listed in Illinois' Statewide Transportation Improvement Program (STIP).

31-7.02(f) Certification

IDOT agrees to follow all control documents, Federal and State laws, regulations, and directives for the design, construction, operation, and maintenance of all Federal-aid projects.

FHWA is not precluded from reviewing or investigating any phase of the Federal-aid program including control documents or any Federal-aid projects, especially those that contain unique features or those with unusual circumstances. Furthermore, this agreement does not preclude IDOT requesting FHWA involvement in projects. This agreement does not change any of the responsibilities of FHWA regarding the requirements of the NEPA, Title VI of the Civil Rights Acts, Fair Housing Act, and the Uniform Relocation Assistance and Land Acquisition Policies Act.

31-8 ADHERENCE TO DESIGN CRITERIA

Parts IV, Roadway Design Elements, and V, Design of Highway Types, present literally thousands of pieces of information on geometric design for application on individual projects. In general, the designer is responsible for making every reasonable effort to meet these criteria in the project design. However, it will not always be practical to meet the IDOT criteria. Therefore, this section presents IDOT's procedures for the appropriate action when the design criteria are not met.

31-8.01 Department Intent

The general intent of the Illinois Department of Transportation is that all road design criteria in Parts IV and V typically should be met and that, wherever practical, the proposed design should exceed the lower criteria. In addition, where a range of values is presented, the designer should make every reasonable effort to provide a design that is near the desirable or preferred value. This is intended to ensure that the Department will provide a highway system that meets the transportation needs of the State and provides a reasonable level of safety, comfort, and convenience for the traveling public. However, recognizing that this will not always be practical or cost effective, the Department has established a process to evaluate and approve exceptions to geometric design criteria.

31-8.02 Design Criteria Checklist

To ensure that designers have considered and evaluated any design exceptions to Department projects, a "Design Criteria Checklist" has been developed to document the review. This Checklist should be completed for each new construction, reconstruction, 3R non-freeway, or 3R freeway project. See the Appendix to Chapter 31 for a copy of the Checklist. The completed Checklist should be included in the permanent project file and used in conjunction with Phase I engineering reports as discussed in Chapter 12. The results from the Checklist should be discussed at the district coordination meetings.

31-8.03 Hierarchy of Design Criteria

The design criteria in the *BDE Manual* have varying levels of importance. Therefore, the Department has established the following hierarchy of importance for the IDOT design criteria.

31-8.03(a) Level One Design Exceptions

Level One Design Exceptions include the controlling design criteria established by FHWA and the disabled accessibility criteria. These criteria are judged to be those design elements that are the most critical indicators of a highway's safety and its overall serviceability. The "Level One Design Criteria Checklist" identifies those design elements in the Level One category. IDOT uses its district coordination meetings for discussing, evaluating, documenting, and/or approving design exceptions to Level One criteria. See Section 31-8.04.

31-8.03(b) Level Two Design Exceptions

Level Two Design Exceptions include additional important indicators of a highway's safety and serviceability but are not considered as critical as the Level One criteria. When Level Two criteria are not met, the designer must discuss these at the district coordination meetings. Usually, less detailed documentation is needed to justify the decision. The "Level Two Design Criteria Checklist" identifies those design elements in the Level Two category.

31-8.04 <u>Design Exception Process</u>

31-8.04(a) IDOT Procedures

The design exception process applies to all capital improvement projects considered new construction, reconstruction, 3R, 3P, or SMART. Design exceptions are discussed at project coordination meetings held in each district office. These meetings are usually scheduled monthly and are attended by representatives from FHWA and BDE.

During these meetings, the district discusses design details of projects in the annual and multi-year programs and, for each project, discusses and provides justification for the need for design exceptions to Department design criteria. BDE and FHWA can typically inform the district if a design exception can be granted. Any agreements reached at the meeting are documented by minutes prepared for each project, which are included in the Phase I engineering report. When design approval is requested, the design exceptions should be identified in the transmittal memo and reference made to their approval at a coordination meeting. On Federal-aid projects, design exceptions are granted through typical IDOT procedures and are usually accepted by FHWA through the FHWA/IDOT Project Oversight Agreement. See Figure 31-7A.

During project development, if the district determines that a major design change is desirable from the approved Phase I engineering report, the proposed change must be coordinated with BDE. The district must prepare either a memo requesting a design change or discuss the proposed design change at a district coordination meeting. Usually, the Regional Field Engineer should be able to approve the design change at a meeting.

31-8.04(b) FHWA Procedures

Because IDOT has a Project Oversight Agreement with FHWA, FHWA's direct involvement in most projects is quite limited. See Figure 31-7A.

If a project will require FHWA design exception approval, this is usually determined at coordination meetings. The project minutes usually provide the necessary documentation of the design exception and the concurrence of the exception. However, on occasion, a project on the Interstate system or on a NHS route with access control may require the preparation of a report and a formal request to FHWA for a design exception.

31-8.04(c) Accessibility Standards for the Disabled

Section 58-1 presents the IDOT application of the Federal standards for accessibility for disabled individuals as promulgated in the *Americans with Disabilities Act* (ADA). The following procedure will apply to a request for a waiver to the accessibility standards:

1. Procedure. ADA provides a waiver procedure that can be used where site conditions and/or topography preclude the use of the ADA standards. However, granting a waiver is extraordinarily rare and, therefore, IDOT should pursue this option only as a last resort.

2. District Coordination Meetings. Any contemplated exceptions to ADA standards should be discussed with FHWA at the district coordination meetings.

3. Documentation. The district must fully document its evaluation of the project site conditions and must clearly demonstrate that a waiver is justified. The content of the waiver request will vary on a case-by-case basis. For example, if the 2% sidewalk cross slope cannot be maintained across several driveways, the following information may be appropriate:

    - a set of plans showing the location of each driveway and the profile grade of the driveway,
    - the station (R) or (L) of centerline of each driveway,
    - the street address for each affected property,
    - the sidewalk cross slope that is proposed across each of the affected driveways,
    - what work would be required to achieve the 2% cross slope, and
    - the cost of achieving the 2% cross slope at each driveway.

4. Submission. The district will submit its documentation to BDE with a request for the waiver to ADA standards.

31-9 REFERENCES

1. *A Policy on Geometric Design of Highways and Streets*, AASHTO, 2001.

2. *Highway Capacity Manual 2000*, Transportation Research Board, 2000.

3. NCHRP 400 *Determination of Stopping Sight Distances*, Transportation Research Board, 1997.

4. "Stopping Sight Distance for Large Trucks," TRR 1208, Transportation Research Board, 1989.

5. FHWA Report No. FHWA-TO-81-1, *A User's Guide to Positive Guidance*, Federal Highway Administration, U.S. Department of Transportation, 1981.

6. *Manual on Uniform Traffic Control Devices*, *Millennium Edition*, FHWA ATSSA, AASHTO, and ITE, 2001.

7. "Design Exceptions: Legal Aspects," TRR 1445, Transportation Research Board, 1994.

Appendix

DESIGN CRITERIA CHECKLIST

This Appendix to Chapter 31 presents the following:

- the Design Criteria Checklist,
- the Level One Design Criteria Checklist, and
- the Level Two Design Criteria Checklist.

Illinois Department of Transportation

DESIGN CRITERIA CHECKLIST

1. **Application**

The designer can use the Level One and Level Two Design Criteria Checklists to summarize compliance with design criteria and assist in the documentation of the adherence of the proposed project design to the design criteria. These checklists become a part of the permanent project file.

2. **Level One Design Exceptions**

A Level One design exception involves one of the controlling design criteria. Check the appropriate boxes on the "Level One Design Criteria Checklist" (p. 3). The determination of whether or not the proposed project design meets the IDOT controlling design criteria is dependent upon the project scope of work. If, for example, a 3R non-freeway project is under design, Chapter 49 will apply. For any Level One element which does not meet IDOT design criteria, the designer should prepare a statement for use at monthly coordination meetings which:

- identifies the design element,
- identifies IDOT design criteria,
- discusses the proposed design, and
- provides justification for the design exception.

The written summary of the discussion at the coordination meeting will document the justification for a design exception. Include the minutes of the meeting describing the project in the Phase I engineering report.

3. **Level Two Design Exceptions**

A Level Two design exception does not involve one of the controlling design criteria. Check the appropriate boxes on pp. 4-10 of the "Design Criteria Checklist." The determination of whether or not the proposed project design meets IDOT design criteria is dependent upon the project scope of work. If, for example, a 3R non-freeway project is under design, Chapter 49 will apply. For any Level Two element which does not meet IDOT design criteria, the designer should prepare a statement similar to that for a Level One exception.

It should be noted that Level Two design exceptions may not require as much justification to receive concurrence of the exception. The written summary of the discussion at the coordination meeting will document the justification for a design exception.

4. **Project Identification**

Federal Project No.: ______________________________
Marked Route No.: ______________________________
Functional Classification: ______________________________
Highway Type: ______________________________
Project Location: ______________________________
____
____

County/City: ______________________________
Project Length: ______________________________

5. **Project Scope of Work**

a. Is project located on the NHS? ☐ Yes ☐ No

b. Check the appropriate box. See Section 31-6 for definitions.

☐ New construction
☐ *Reconstruction
☐ 3R (non-freeway)
☐ *3R (freeway)

c. Provide a brief project description:

____

**Note: May include "Allowed to Remain in Place" criteria.*

6. **Evaluating Exceptions**

When evaluating exceptions to design criteria, the primary considerations are:

- safety,
- capacity,
- compatibility with adjacent sections,
- time to construction of ultimate improvement, and
- construction costs.

7. **District Coordination Meetings**

Has project been discussed at district coordination meetings? ☐ Yes ☐ No

Date:

Date: __________________

Level One Design Criteria Checklist

Sheet 1 of 1

Route: _________________________ Section: _________________ County: __________________

Design Criteria for Mainline Only (Provide numerical value for project, where indicated.)	Does the proposed design meet IDOT criteria?		
	Yes	No*	N/A
1. Design Speed: ______ mph (km/h)			
2. Lane Widths: ______ feet (meters)			
3. Through Travel Lane Cross-Slopes in Percent (%): Lane 1 _______ Lane 2 _______ Lane 3 _______			
4. Shoulder Widths: _______feet (meters) (inside) _______feet (meters) (outside)			
5. Horizontal Curvature (Minimum Radius for selected design speed) __________ feet (meters)			
6. Superelevation Rates (e_{max} = ________ %)			
7. Stopping Sight Distance at Crest Vertical Curves (Level SSD for Passenger Cars)			
8. Stopping Sight Distance at Sag Vertical Curves (Level SSD for Passenger Cars)			
9. Stopping Sight Distance on Inside of Horizontal Curves (Level SSD for Passenger Cars)			
10. Clear Roadway Bridge Widths: _________feet (meters)			
11. Structural Capacity of Bridges: ______________			
12. Vertical Clearances: ________________			
13. Maximum Grades: _________________			
14. Accessibility Criteria for Disabled Persons			

* Justification for any design exceptions must be discussed at monthly coordination meetings held in each district and must be documented in the Phase I report.

Note: Numbers 1, 2, 3, and 4 apply throughout the project. The remaining criteria (e.g., superelevation rates) apply to specific sites within the project limits.

Date: ________________

Level Two Design Criteria Checklist

Route: _____________ Section: ____________________ County: __________________

Design Criteria		Does the proposed design meet IDOT criteria?		
		Yes	No*	N/A
1. Basic Design Controls				
a. Level of Service (mainline)				
b. SSD application at horizontal curves (downgrade adjusted SSD used)	Horz.			
c. SSD application for vertical curves (downgrade adjusted SSD used)	Vert.			
d. Truck SSD (level) (at specific sites)				
2. Horizontal Alignment (Mainline)				
a. Traveled way widening				
b. Superelevation transition lengths				
c. Superelevation distribution between tangent and curve				
d. “Breakover” of outside shoulder on super-elevated curves				
e. Relative longitudinal slope of shoulder to edge of traveled way on high side of S.E. curve adjacent to bridge with S.E.				
f. Superelevation development at reverse curves				
g. Is superelevation transition length located off of bridges and bridge approach pavements?				

*See Section 31-8 of *BDE Manual.*

Design Criteria	Does the proposed design meet IDOT criteria?		
	Yes	No*	N/A
3. Vertical Alignment (Mainline)			
a. Minimum grades considering drainage			
b. Critical length of grade			
c. Warrants for truck-climbing lanes			
d. Design criteria for truck-climbing lanes (e.g., lane width and shoulder width)			
e. Minimum length of vertical curves for selected design speed			
f. Maximum length of vertical curves (drainage of curbed facilities and bridges)			
4. Cross Section Elements (Mainline)			
a. Design of parking lanes: • Cross-slope_______% • Width _______ feet (meters)			
b. Design of sidewalks: • Cross-slope _______ % • Width _______ feet (meters) • Longitudinal slopes _______ %			
c. Type of curb and gutter used on median:			
d. Drainage of raised curb medians: • Direction of flow of median surface or pavement ____________________ • Direction of cross-slope on gutter ___ %			
e. Type of curb and gutter used along outside edges of pavement _________________			
f. TWLTL width: • Flush type _______ feet (meters) • Traversable type _______ feet (meters)			

*See Section 31-8 of *BDE Manual.*

Design Criteria	Does the proposed design meet IDOT criteria?		
	Yes	No*	N/A
g. Median widths: • Urban _______ feet (meters) • Suburban _______ feet (meters) • Rural _______ feet (meters)			
h. Shoulder cross slopes ________ %			
i. Fill slopes: _____________ (V:H)			
j. Outside roadway ditch: • Slopes _______ • Depth _______ • Widths _______ Median ditch: • Widths _____ • Slopes ______ • Depth ______			
k. Cross-section transitions into bridges/ underpasses			
l. Use of mountable curbs (V > 45 mph (70 km/h))			
m. Cross-section transition details (e.g., four-lane to two-lane)			
n. Design of frontage roads: • Des. speed ___ • Pvmt. width ____ • Shld. width____ • Cross-slopes ___ • Super. rate ____ • Ditch slopes ____			
5. Roadside Safety			
a. Horizontal clearances: • Clear zones on tangent sections • Clear zones on outside of horizontal curves			
b. Barrier warrants			
c. Barrier length of need			
d. Deceleration criteria for impact attenuators			

*See Section 31-8 of *BDE Manual.*

Design Criteria		Does the proposed design meet IDOT criteria? Yes	No*	N/A
6. Intersections				
a. Accommodation of design vehicle (Identify Vehicle) ____________				
b. Level of service: • Through Lanes ____________ • Turn Lanes ____________				
c. Skew angle				
d. Profiles				
e. Volume guidelines for turn-lanes: • Right-turns • Left turns				
f. Design of right-turn lanes				
Design of left-turn lanes				
g. Turn-lane tapers	Approach Taper			
	Departure Taper			
	Bay Taper			
h. Turning roadway widths				
i. Turn-lane lengths	Deceleration (Rural)			
	Storage (Urban)			
j. Intersection sight distance: List criteria and type: ____________ ____________				
k. Median opening length: ____________				
l. Minimum corner island size: ____________				
m. Does right-turn radius accommodate design vehicle without encroachment?				
n. Driveway widths				

*See Section 31-8 of *BDE Manual.*

Design Criteria		Does the proposed design meet IDOT criteria?		
		Yes	No*	N/A
o. Type of traffic control: • Two-way stop • All-way stop • Traffic signals				
p. Is maximum grade exceeded on any approach?				
q. Max “e” for intersections on curve				
7. Interchanges				
a. Exit Terminal	Standard Type			
	Design speed of first curve			
	Are any exit terminals located on mainline horizontal curve?			
b. Entrance Terminal	Standard Type			
	Length of tangent after the entering curve			
	Design speed of entering curve			
c. Design speed of ramp proper: ____________________ mph (km/h)				
d. Design speed of crossroad: ____________________ mph (km/h)				
e. Maximum ramp grades: • Exit ramp ____________ % • Entrance ramp ____________ %				
f. Ramp pavement width				
g. Ramp shoulder widths: • Left ____________ • Right ____________				
h. Horizontal ramp curvature in conjunction with selected deign speeds				

*See Section 31-8 of *BDE Manual.*

Design Criteria		Does the proposed design meet IDOT criteria? Yes	No*	N/A
i. Superelevation development on ramps	Superelevation Rate			
	Transition Length			
	Distribution Between Tangent & Curve			
j. Vertical curvature compliance with selected design speed on ramp				
k. Length of access control at crossroad				
l. Type of traffic control at crossroad: • Stop signs • Traffic signals • Free flow				
m. Is length of crest vertical curve used on crossroad ≥ that required by the selected design speed of crossroad?				
n. Are crossroad approach grades through ramp/ crossroad intersections ≤ 2%?				
o. Are ramp/crossroad intersections located on a tangent section of crossroad alignment?				
p. Is decision sight distance available in advance of exit gore?				
q. Is clear recovery area available beyond gore nose?				
r. Level of service: • Exit terminal ____________ • Entrance terminal ____________ • Ramp proper ____________ • Weaving area ____________ • Ramp/crossroad intersection ______				

*See Section 31-8 of *BDE Manual.*

Design Criteria			Does the proposed design meet IDOT criteria?		
			Yes	No*	N/A
s. Freeway lane drops	Location	Upgrade			
		Downgrade			
		Inside Lane			
		Outside Lane			
		At Exit Terminal			
		Beyond Exit Terminal			
	Taper Length				

Prepared By: ______________________________________
Designer (IDOT or Consultant)

*See Section 31-8 of *BDE Manual.*